



Friday, June 4, 2021

City of Aurora, Colorado  
Engineering Division  
15151 E Alameda Pkwy  
Aurora, CO 80012

RE: LOT 1, BLOCK 2, TOWER CENTER FOR INDUSTRY SUBDIVISION FILING NO. 2 PRELIMINARY DRAINAGE COMPLIANCE LETTER

City of Aurora Engineering Division:

Two new buildings are proposed at the southwest corner of 22<sup>nd</sup> Avenue and Tower Road in Aurora, Colorado. This parcel is known as Lot 1, Block 2, Tower Center for Industry Subdivision Filing No. 2. A 58,000 square-foot building exists on the site currently. Proposed development includes the addition of a 17,000 square-foot office building, a 5,000 square-foot storage building, and associated parking and utilities. The site was previously studied by R&R Engineers-Surveyors, Inc. in a report titled, "Final Drainage Report for Specialty Wood Products, Lot 1, Block 2, Town Center for Industry Subdivision Filing No. 2, Aurora, CO"; approved for construction on September 20, 2006 by the City of Aurora Water Department and City Engineer (COA Approval 206174). As part of that private development, improvements were constructed including a detention pond, underground utilities, parking, and the building that now all exist on the site. That development was also in conformance with the Master Drainage Report for this area. (COA Approval C8-2-1320) The site was also studied in a previous drainage compliance letter (COA Approval 221123FD1) dated February 5, 2021. That study covered an expansion to the existing building, analyses for curb chases from the building out to 22<sup>nd</sup> Avenue, and calculations showing that the existing detention pond meets Aurora's updated detention criteria.

Compared with the earlier report, the boundaries of several basins are modified. Basin C becomes basins C1 and C2 to demonstrate that some of the storm water is diverted past the north end of the proposed storage building. A new riprap rundown will convey water from Basin C1 into the pond.

The boundary between basins F and H is adjusted as well. In Basin F, storm water currently runs from an existing curb chase down the east portion of the property and then west in a wide valley pan to the detention pond. The valley pan will be removed and these waters will be conveyed along a curb near the front of the office building.

Basin G now includes the proposed building and some landscaped area east and south of the building. Rainfall on the building will be captured and diverted underground by roof drains. The roof drains direct the water to the detention pond. For the landscaped area east and south of the building, landscape inlets capture the storm water and connect to the same underground storm sewer that conveys the roof flows.

The detention pond was shown in the previous report to meet criteria with a site that was 66% impervious. With the development proposed at this time, the tributary site's imperviousness will decrease to less than 62%. Therefore, the pond has sufficient capacity based on those previous analyses.

City of  
Aurora, Colorado  
Engineering Division  
LOT 1, BLOCK 2, TOWER CENTER FOR INDUSTRY SUBDIVISION FILING NO. 2  
DRAINAGE COMPLIANCE LETTER  
Friday, June 4, 2021  
Page 2 of 2



By graphical inspection, the subject property is situated in Unshaded Flood Zone "X", the area determined to be outside the 0.2% annual chance flood plain according to Flood Insurance Rate Map (FIRM) community panel No. 08005C0182K last revised December 17, 2010. The site lies approximately 3,500 feet east of the outfall into Sand Creek.

You may reach me by phone at (303) 337-1393 or via email at [canderson@engineeringserviceco.com](mailto:canderson@engineeringserviceco.com).

Sincerely,

A handwritten signature in blue ink, appearing to read "Chad M. Anderson", is written over a light blue rectangular background.

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Chad M. Anderson, P.E.  
Project Manager  
Engineering Service Company  
Registered P.E. No. 55094

**Table—Hydrologic Soil Group**

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
TuB	Truckton sandy loam, 0 to 3 percent slopes	A	3.1	31.5%
VoB	Vona sandy loam, 1 to 3 percent slopes	A	6.7	68.5%
<b>Totals for Area of Interest</b>			<b>9.8</b>	<b>100.0%</b>

**Rating Options—Hydrologic Soil Group**

*Aggregation Method: Dominant Condition*

*Component Percent Cutoff: None Specified*

*Tie-break Rule: Higher*

**TABLE 1**  
**RUNOFF COEFFICIENTS AND PERCENTS IMPERVIOUS**

LAND USE OR SURFACE CHARACTERISTICS	PERCENT IMPERVIOUS	FREQUENCY			
		2	5	10	100
<u>Business:</u>					
Commercial Areas	95	.87	.87	.88	.89
Neighborhood Areas	85	.60	.65	.70	.80
<u>Residential:</u>					
Single-Family (**)	(*)	.40	.45	.50	.60
Multi-Unit (detached)	60	.45	.50	.60	.70
Multi-Unit (attached)	75	.60	.65	.70	.80
1/2 Acre Lot or Larger	(*)	.30	.35	.40	.60
Apartments	80	.65	.70	.70	.80
<u>Industrial:</u>					
Light Areas	80	.71	.72	.76	.82
Heavy Areas	90	.80	.80	.85	.90
<u>Parks, Cemeteries</u>	5	.10	.10	.35	.60
<u>Playgrounds</u>	10	.15	.25	.35	.65
<u>Schools</u>	50	.45	.50	.60	.70
<u>Railroad Yard Areas</u>	15	.40	.45	.50	.60
<u>Undeveloped Areas:</u>					
Historic Flow Analysis, Greenbelts, Agricultural	2	(See "Lawns")			
Off-Site Flow Analysis (when land use not defined)	45	.43	.47	.55	.65

**TABLE 1** (continued)

**RUNOFF COEFFICIENTS AND PERCENTS IMPERVIOUS**

LAND USE OR SURFACE CHARACTERISTICS	PERCENT IMPERVIOUS	FREQUENCY			
		2	5	10	100
<u>Streets:</u>					
Paved	100	.87	.88	.90	.93
Gravel	40	.15	.25	.35	.65
<u>Concrete Drive and Walks</u>	96	.87	.87	.88	.89
<u>Roofs</u>	90	.80	.85	.90	.90
<u>Lawns, Sandy Soil (A and B Soils):</u>	2				
2% Slope		.05	.06	.08	.10
2-7% Slope		.10	.11	.13	.15
>7% Slope		.15	.16	.18	.20
<u>Lawns, Clay Soil (C and D Soils):</u>	5				
2% Slope		.13	.14	.15	.17
2-7% Slope		.18	.19	.20	.22
>7% Slope		.25	.27	.30	.35

NOTE: These Rational Formula coefficients may not be valid for large basins

(\*)See Figures RO-3 through RO-5 of USDCM Volume 1 for percent impervious.

(\*\*)Up to 5 units per acre. Single-family with more than 5 units per acre, use values for multi-unit/detached

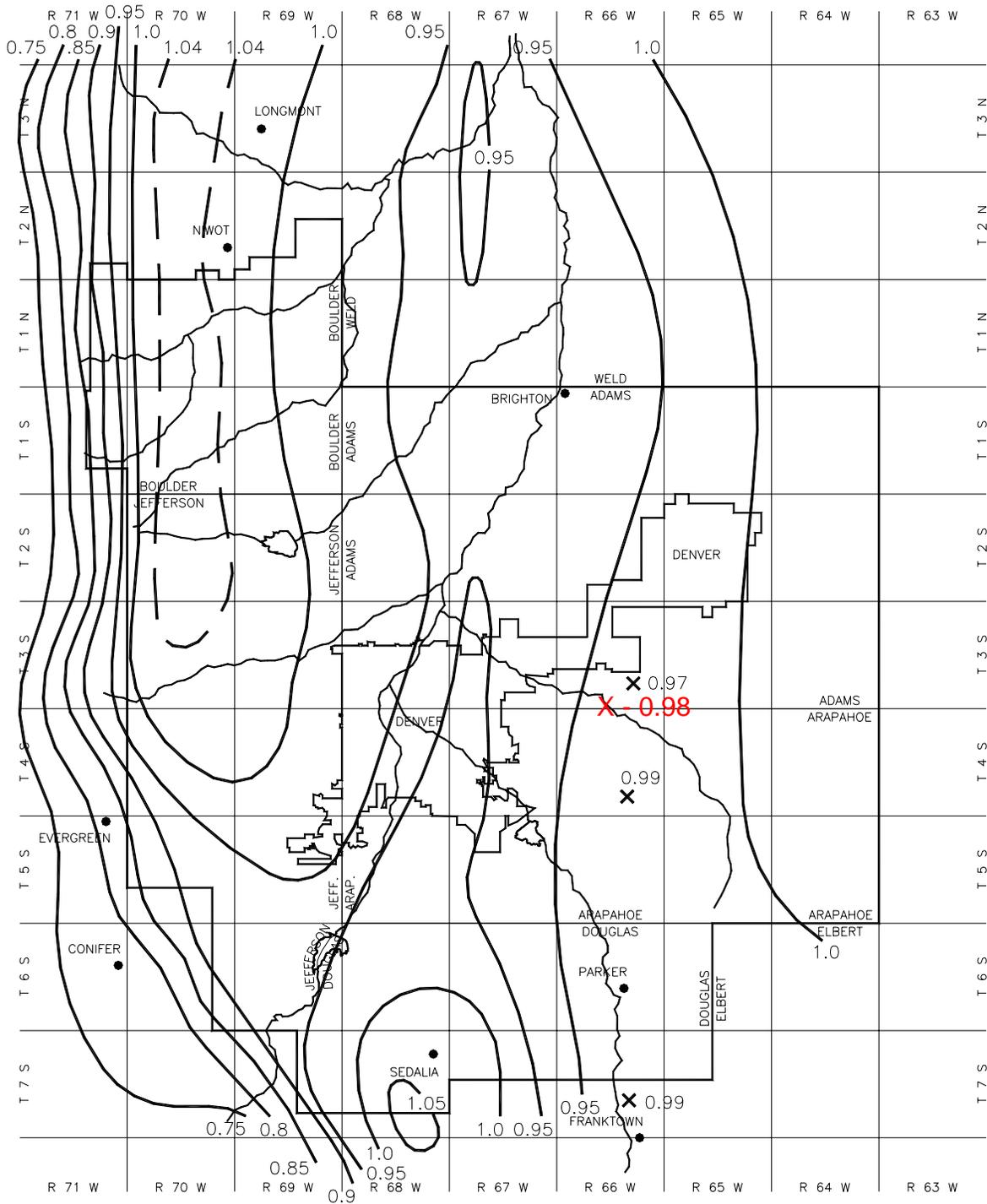


Figure RA-1—Rainfall Depth-Duration-Frequency: 2-Year, 1-Hour Rainfall

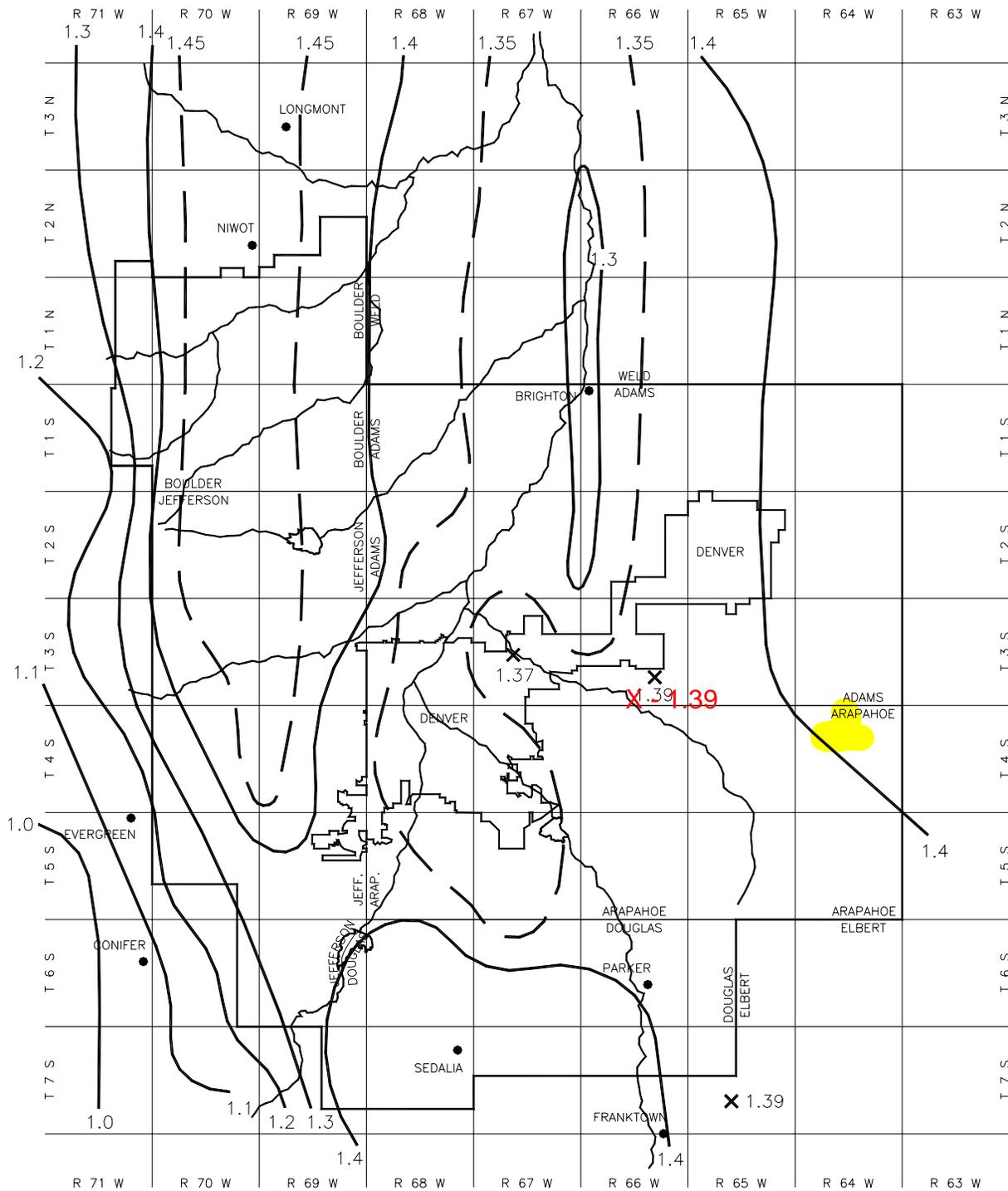


Figure RA-2—Rainfall Depth-Duration-Frequency: 5-Year, 1-Hour Rainfall

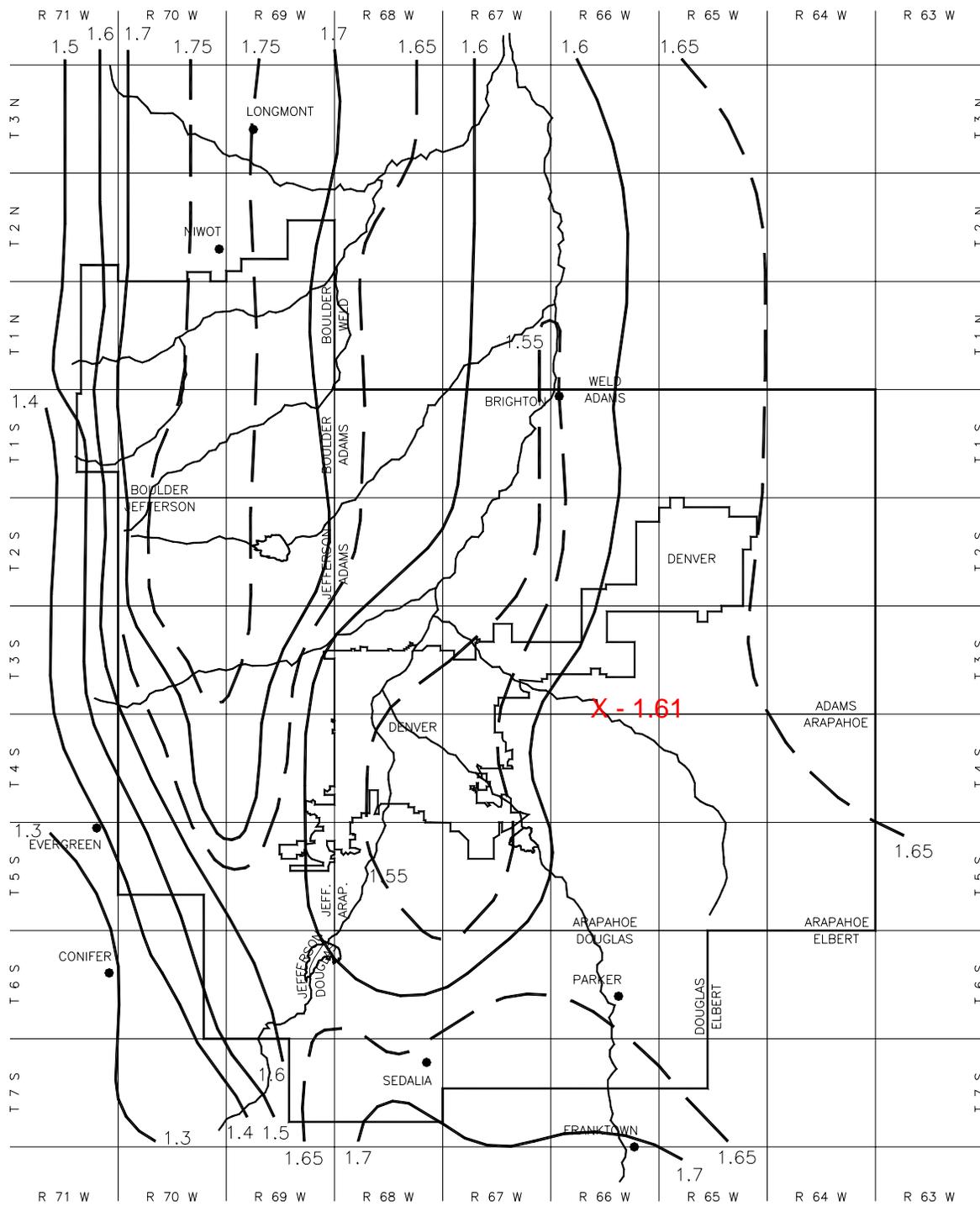


Figure RA-3—Rainfall Depth-Duration-Frequency: 10-Year, 1-Hour Rainfall

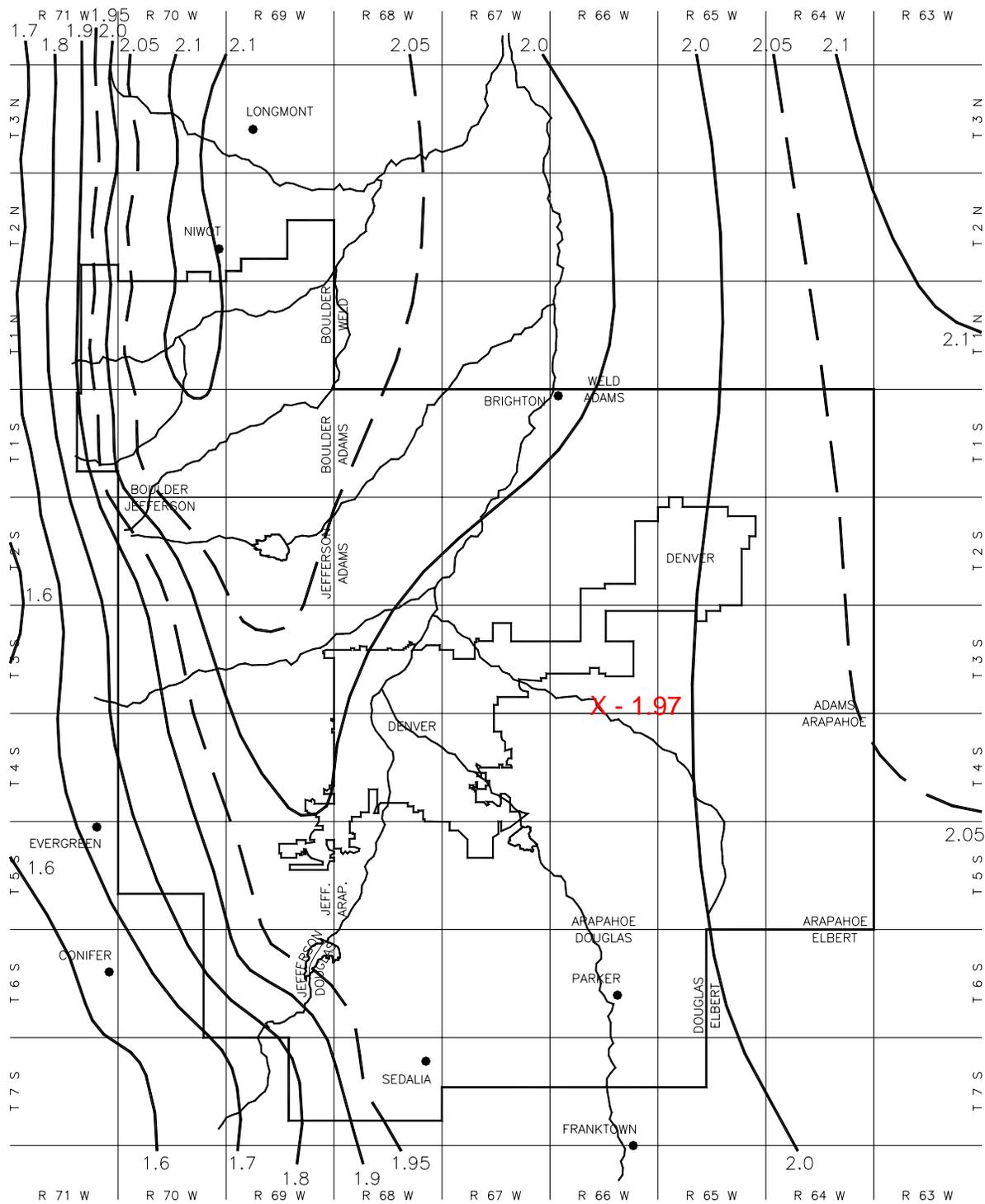


Figure RA-4—Rainfall Depth-Duration-Frequency: 25-Year, 1-Hour Rainfall

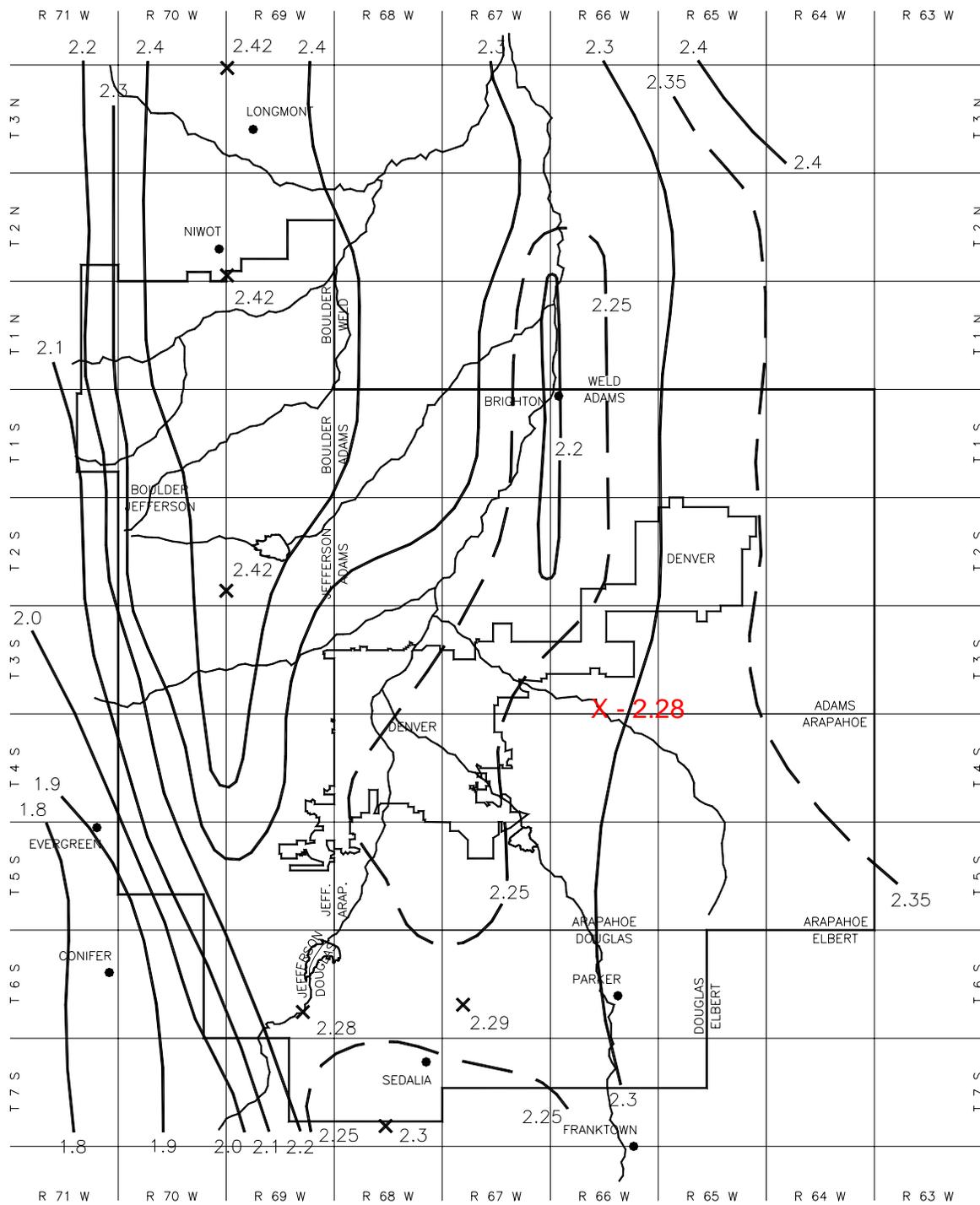


Figure RA-5—Rainfall Depth-Duration-Frequency: 50-Year, 1-Hour Rainfall

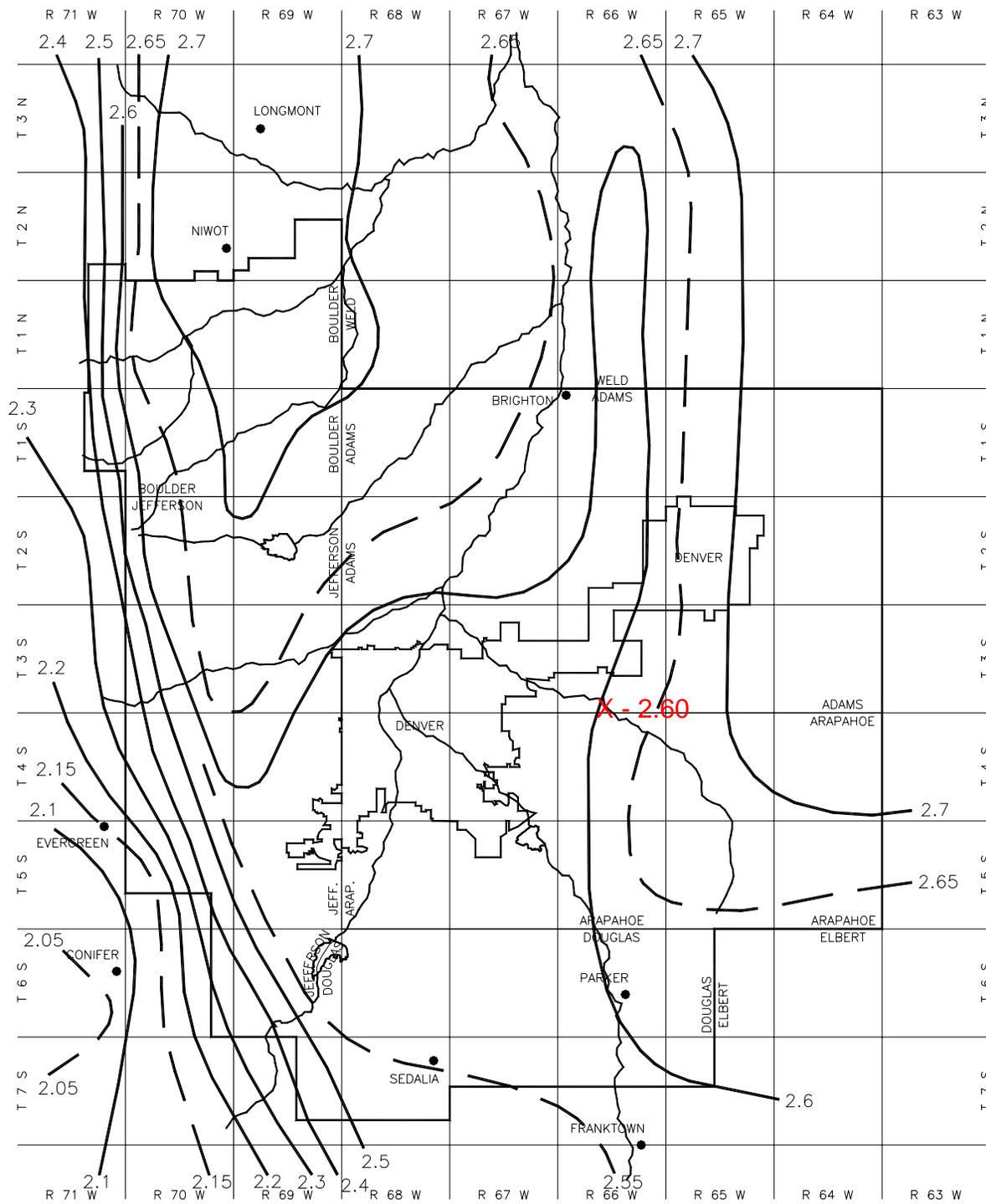


Figure RA-6—Rainfall Depth-Duration-Frequency: 100-Year, 1-Hour Rainfall

## **APPENDIX B**

- A. Developed Condition Hydrologic Conditions
- B. Detention Pond Analysis and Design

# Composite Basin Parameter Calculations

Job Name: T-Mobile Switch Expansion

Date: 2021-06-04

By: CMA

## Imperviousness and "C" Values

	I	C <sub>2</sub>	C <sub>5</sub>	C <sub>100</sub>
Roofs	90.00%	0.80	0.85	0.90
Lawn, 2-7% Slope, A&B Soils	2.00%	0.15	0.16	0.20
(Asphalt) Drives & Walks	100.00%	0.87	0.88	0.93

Ref. Aurora Storm Drainage Design and Technical Criteria, Table 1

Basin	Roof	Lawn	Drives & Walks	Total	I	Weighted Runoff Coeff		
						C <sub>2</sub>	C <sub>5</sub>	C <sub>100</sub>
A1	0.20	0.18	0.03	0.41	53.10%	0.53	0.56	0.60
A2	0.21	0.09	0.02	0.33	65.63%	0.62	0.66	0.70
B	0.91	0.00	0.00	0.91	90.00%	0.80	0.85	0.90
C1	0.00	0.06	0.69	0.75	91.59%	0.81	0.82	0.87
C2	0.14	0.04	0.30	0.48	89.71%	0.80	0.82	0.87
D	0.00	0.26	0.13	0.39	34.29%	0.39	0.40	0.44
E	0.00	0.55	0.04	0.60	9.05%	0.20	0.21	0.25
F	0.00	0.27	1.28	1.55	83.19%	0.75	0.76	0.80
G	0.00	0.23	1.04	1.27	82.05%	0.74	0.75	0.80
H	0.40	0.23	0.07	0.70	62.08%	0.59	0.63	0.67
OS-1	0.15	1.84	0.59	2.58	29.53%	0.35	0.36	0.41
<b>Total</b>	<b>2.02</b>	<b>3.75</b>	<b>4.19</b>	<b>9.97</b>	<b>56.75%</b>	<b>0.58</b>	<b>0.60</b>	<b>0.65</b>
<b>Total Into Pond</b>	<b>1.61</b>	<b>3.48</b>	<b>4.14</b>	<b>9.23</b>	<b>61.29%</b>	<b>0.59</b>	<b>0.60</b>	<b>0.65</b>

(Includes all except A1 and A2)

# Time of Concentration Calculations

Job Name: T-Mobile Switch Expansion

Date: 2021-06-04

By: CMA

Basin Data			*Initial/Overland Flow Time (Ti)			Travel Time (Tt)				**Tc Check Urbanized Basin		Final Tc	Remarks
Desig	C <sub>5</sub>	Area Ac	Length Ft	Slope %	Ti Min	Length Ft	Slope %	***Vel FPS	Tt Min	Ti+Tt	Tc Max	Min	
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)	(13)	
A1	0.53	0.41	36	3.50	4.10	750	1.00	2.00	6.25	10.35	28.15	10.35	
A2	0.62	0.33	36	3.50	3.44	3200	1.00	2.00	26.67	30.11	61.70	30.11	
B	0.80	0.91	300	1.00	9.38	44	1.00	2.00	0.37	9.75	22.58	9.75	
C1	0.81	0.75	170	2.50	5.08	219	1.00	2.00	1.83	6.90	22.01	6.90	
C2	0.80	0.48	181	4.00	4.68	135	1.00	2.00	1.13	5.81	20.50	5.81	
D	0.39	0.39	117	1.50	12.18	0	1.50	2.45	0.00	12.18	19.26	12.18	
E	0.20	0.60	118	2.00	14.01	0	2.00	2.83	0.00	14.01	19.12	14.01	
F	0.75	1.55	300	2.00	8.79	115	2.00	2.83	0.68	9.47	21.90	9.47	
G	0.74	1.27	300	2.00	9.00	247	2.50	3.16	1.30	10.30	22.91	10.30	
H	0.59	0.70	300	2.50	11.71	10	3.00	3.46	0.05	11.76	20.59	11.76	
OS-1	0.35	2.58	300	2.00	18.60	184	2.50	3.16	0.97	19.57	22.46	19.57	

\*Ti calculated from City of Aurora Storm Drainage Criteria Manual, Equation 5.3

\*\*Tc calculated from City of Aurora Storm Drainage Criteria Manual, Equation 5.4

$$t_i = \frac{0.395(1.1 - C_5)\sqrt{L}}{\sqrt{S}} \quad (5.3)$$

where  $t_i$  = initial or overland flow time (minutes)  
 $C_5$  = runoff coefficient for 5-year frequency  
 $L$  = length of overland flow, (ft., 500 ft. max.)  
 $S$  = average basin slope (ft/ft)

$$t_c = \frac{L'}{180} + 10 \quad (5.4)$$

Where  $t_c$  = time of concentration (minutes)

$L'$  = length of flow to first design point from the most remote point (feet)

# Stormwater Runoff

Job Name: T-Mobile Switch Expansion

Date: 2021-06-04 Project:

Calculated by: CMA

Design Storm:

2-yr

1-hr runoff depth

0.97 in

Design Point	Direct Runoff							Total Runoff				Remarks
	Basin ID	Area (Ac)	Runoff Coeff	Tc (min)	CA	I <sup>1</sup> (in/hr)	Q (cfs)	Tc (min)	Total CA	I <sup>1</sup> (in/hr)	Q (cfs)	
1	A1	0.41	0.53	10.35	0.22	2.59	0.56					Flows North along Tower Road.
10	A2	0.33	0.62	30.11	0.20	1.52	0.31					Flows west along 22nd.
2	B	0.91	0.80	9.75	0.73	2.65	1.94					Flows into Pond.
3	C1	0.75	0.81	6.90	0.61	3.00	1.82					Flows into Pond.
10	C2	0.48	0.80	5.81	0.38	3.16	1.21					Flows into Pond.
4	D	0.39	0.39	12.18	0.15	2.42	0.36					Flows into Pond.
5	E	0.60	0.20	14.01	0.12	2.27	0.27					Flows into Pond.
6	F	1.55	0.75	9.47	1.15	2.68	3.09	10.30	2.82	2.59	7.32	B, G, and F. Flows into Pond.
7	G	1.27	0.74	10.30	0.94	2.59	2.44	10.30	1.67	2.59	4.33	B and G. Flows into Pond.
8	H	0.70	0.59	11.76	0.42	2.46	1.02					Flows into Pond.
9	OS-1	2.58	0.35	19.57	0.91	1.93	1.76					Flows into Pond.

\*Intensity (I) calculated using COA Intensity equation 5.5

$$I = \frac{28.5 P_1}{(10 + T_c)^{0.786}} \quad (5.5)$$

Where:

I = rainfall intensity (inches per hour)

P<sub>1</sub> = one-hour rainfall depth (inches) from Figures RA-1 through RA-6 in USDCM, Volume 1

T<sub>c</sub> = time of concentration (minutes).

# Stormwater Runoff

Job Name: T-Mobile Switch Expansion

Date: 2021-06-04 Project:

Calculated by: CMA

Design Storm:  
1-hr runoff depth

100-Yr  
2.6 in

Design Point	Direct Runoff							Total Runoff				Remarks
	Basin ID	Area (Ac)	Runoff Coeff	Tc (min)	CA	I <sup>1</sup> (in/hr)	Q (cfs)	Tc (min)	Total CA	I <sup>1</sup> (in/hr)	Q (cfs)	
1	A1	0.41	0.60	10.35	0.25	6.94	1.72					Flows North along Tower Road.
10	A2	0.33	0.70	30.11	0.23	4.07	0.93					Flows west along 22nd.
2	B	0.91	0.90	9.75	0.82	7.10	5.85					Flows into Pond.
3	C1	0.75	0.87	6.90	0.65	8.03	5.23					Flows into Pond.
10	C2	0.48	0.87	5.81	0.42	8.46	3.54					Flows into Pond.
4	D	0.39	0.44	12.18	0.17	6.49	1.10					Flows into Pond.
5	E	0.60	0.25	14.01	0.15	6.09	0.92					Flows into Pond.
6	F	1.55	0.80	9.47	1.24	7.18	8.94	10.30	3.08	6.95	21.41	B, G, and F. Flows into Pond.
7	G	1.27	0.80	10.30	1.01	6.95	7.04	10.30	1.84	6.95	12.76	B and G. Flows into Pond.
8	H	0.70	0.67	11.76	0.47	6.58	3.11					Flows into Pond.
9	OS-1	2.58	0.41	19.57	1.05	5.17	5.45					Flows into Pond.

\*Intensity (I) calculated using COA Intensity equation 5.5

$$I = \frac{28.5 P_1}{(10 + T_c)^{0.786}} \quad (5.5)$$

Where:

I = rainfall intensity (inches per hour)

P<sub>1</sub> = one-hour rainfall depth (inches) from Figures RA-1 through RA-6 in USDCM, Volume 1

T<sub>c</sub> = time of concentration (minutes).



R&R ENGINEERS - SURVEYORS, INC.

206174  
03m  
Rpt-206174  
1981-3044

**FINAL DRAINAGE  
REPORT**

FOR

**SPECIALTY WOOD PRODUCTS  
LOT 1, BLOCK 2, TOWN CENTER FOR INDUSTRY SUBDIVISION  
FILING No. 2  
AURORA, COLORADO**

PREPARED FOR:

**CMC GROUP, INC.  
COLORADO CENTER, TOWER 1  
2000 S. COLORADO BLVD. SUITE 10500  
DENVER, CO 80222  
(303) 741-4500  
CONTACT: GRAHAM BENES**

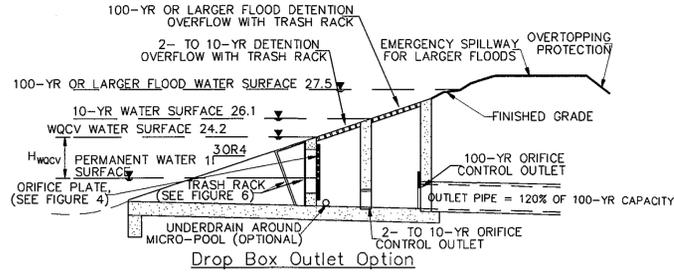
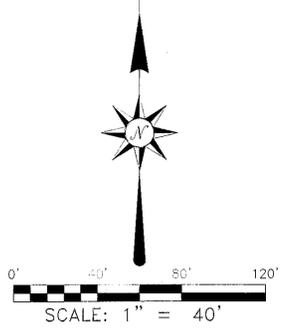
PREPARED BY:

**R & R ENGINEERS - SURVEYORS, INC.  
710 W. COLFAX AVE.  
DENVER, CO 80204  
CONTACT: COLE C. HABERER  
(303) 753-6730  
JOB # CM06002**

AUGUST 9, 2006

APPROVED FOR ONE YEAR FROM THIS DATE	
<i>09.20.06</i>	
<i>Joseph S. Wray</i> Water Department	<i>9-07-06</i> Date
<i>Ronald Degehart</i> City Engineer	<i>9-8-06</i> Date

206129



**NOTE:**  
 1. OWNER/CONTRACTOR MUST OBTAIN N.P.D.E.S. STORM WATER DISCHARGE PERMIT FROM COLORADO DEPARTMENT OF HEALTH IF REQUIRED.  
 2. ALL STORM PIPE IS PRIVATE UNLESS OTHERWISE STATED (PUBLIC).  
 3. PRIVATE STORM PIPE AND INLETS ARE DESIGNED FOR THE 100-YEAR STORM.

CITY OF AURORA PLAN REVIEW IS ONLY FOR GENERAL CONFORMANCE WITH CITY OF AURORA DESIGN CRITERIA AND THE CITY CODE. THE CITY IS NOT RESPONSIBLE FOR THE ACCURACY AND ADEQUACY OF THE DESIGN, OF DIMENSIONS, AND ELEVATIONS, WHICH SHALL BE CONFIRMED AND CORRELATED AT THE JOB SITE. THE CITY OF AURORA THROUGH THE APPROVAL OF THIS DOCUMENT ASSUMES NO OTHER RESPONSIBILITY OTHER THAN AS STATED ABOVE FOR COMPLETENESS AND/OR ACCURACY OF THIS DOCUMENT.

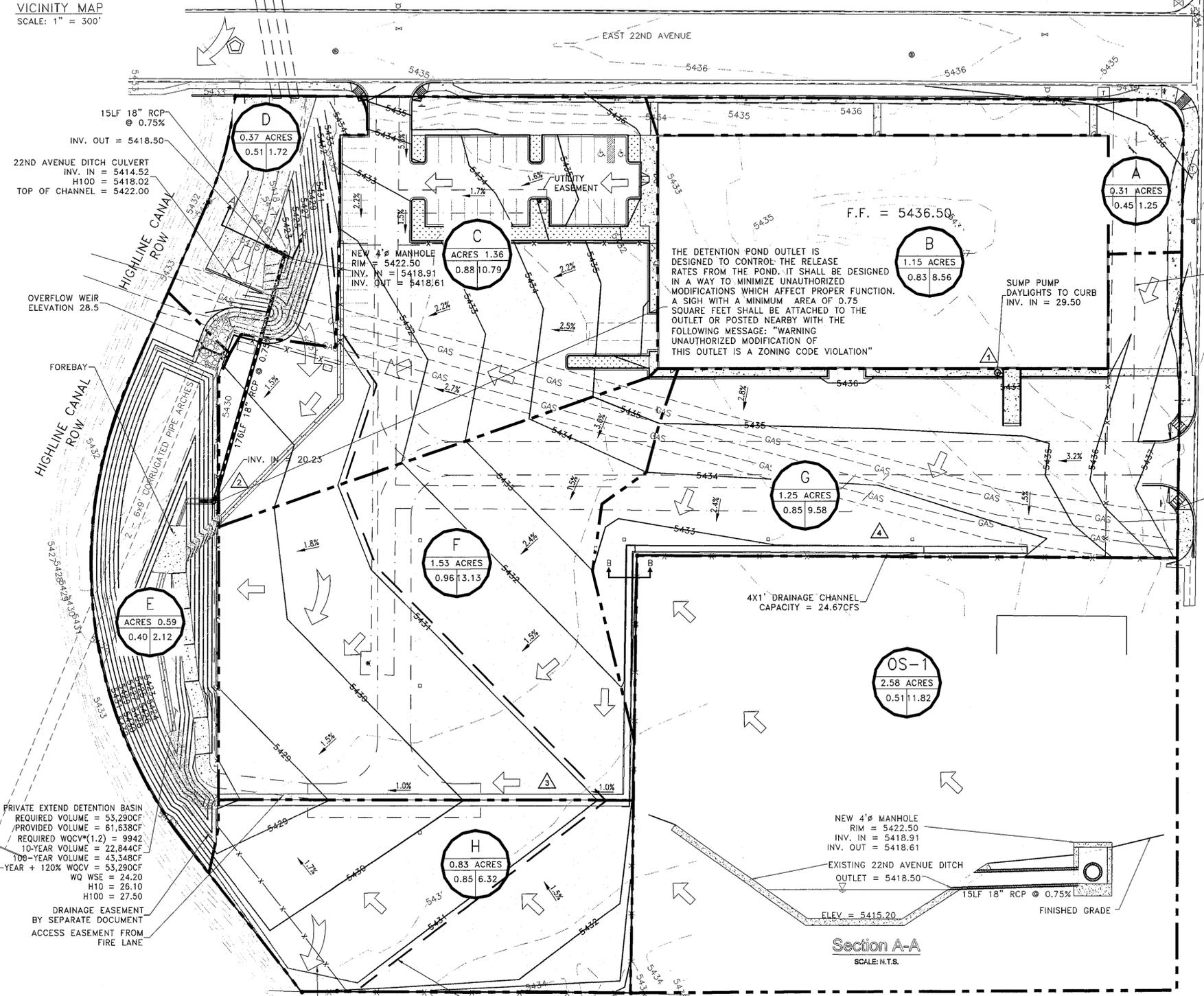
**FLOOD NOTE:** BASED ON A REVIEW OF FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 080002 0185 E PANEL 185 OF 510, DATED AUGUST 16, 1995, THIS PROPERTY IS NOT WITHIN THE 100-YEAR FLOOD PLAIN LIMITS.

**BENCHMARK - COA 12-024A:** CHISELED SO CENTER WEST EDGE CO INLET AT NORTH PCR NORTHEAST CORNER OF RIFLE STREET AND EAST 22ND AVENUE. ELEVATION = 5412.295.

RUNOFF TABLE					
BASIN	DESIGN POINT	AREA (ACRES)	Q <sub>2</sub> (CFS)	Q <sub>100</sub> (CFS)	C <sub>100</sub>
A	N/A	0.31	0.17	1.25	0.45
B	1	1.15	2.79	8.56	0.83
C	2	1.36	3.53	10.79	0.88
D	N/A	0.37	0.32	1.72	0.51
E	N/A	0.59	0.18	2.12	0.40
F	3	1.53	4.50	13.13	0.96
G	4	1.25	3.06	9.58	0.85
H	3	0.83	2.02	6.32	0.85
OS-1	N/A	2.58	1.80	11.82	0.51

**LEGEND**

- PROPERTY LINE
  - 5481 — NEW CONTOUR
  - 5481 --- EXISTING CONTOUR
  - SS --- EXISTING SEWER
  - W --- EXISTING WATER
  - ST --- EXISTING STORM
  - NEW CURB
  - NEW FLOW LINE
  - NEW PAN/LIP
  - EXISTING CURB
  - EXISTING FLOW LINE
  - EXISTING PAN/LIP (IF APPLICABLE)
  - NEW FIRE HYDRANT
  - EXISTING FIRE HYDRANT
  - NEW STORM MANHOLE
  - EXISTING STORM MANHOLE
  - NEW STORM INLET
  - EXISTING STORM INLET
  - NEW DOWN SPOUT
  - NEW NDS GRATE
  - EXISTING NDS GRATE
  - NEW SEWER MANHOLE
  - EXISTING SEWER MANHOLE
  - NEW CLEAN OUT
  - EXISTING CLEAN OUT
  - 2% — SLOPE
  - A — BASIN CALLOUT
  - 1.27 ACRES — BASIN SIZE IN ACRES
  - 0.69 | 6.50 — 100 YEAR FLOW (CFS)
  - — 100-YR COMPOSITE "C" VALUE
  - BASIN BOUNDARY
  - DIRECTION OF FLOW
  - EMERGENCY OVERFLOW
  - NEW CONCRETE
  - NEW ASPH
  - TYPE "M" RIP RAP
  - DETENTION POND LIMITS
- \* NOTE: SYMBOLS MAY NOT BE TO SCALE FOR BETTER GRAPHICAL REPRESENTATION.



VICINITY MAP  
SCALE: 1" = 300'

15LF 18" RCP @ 0.75%  
INV. OUT = 5418.50

22ND AVENUE DITCH CULVERT  
INV. IN = 5414.52  
H100 = 5418.02  
TOP OF CHANNEL = 5422.00

OVERFLOW WEIR  
ELEVATION 28.5

FOREBAY  
HIGHLINE CANAL

PRIVATE EXTEND DETENTION BASIN  
REQUIRED VOLUME = 53,290CF  
PROVIDED VOLUME = 61,538CF  
REQUIRED WQCV\*(1.2) = 9942  
10-YEAR VOLUME = 22,844CF  
100-YEAR VOLUME = 43,349CF  
100-YEAR + 120% WQCV = 53,290CF  
WQ WSE = 24.20  
H10 = 26.10  
H100 = 27.50

DRAINAGE EASEMENT BY SEPARATE DOCUMENT  
ACCESS EASEMENT FROM FIRE LANE

THE DETENTION POND OUTLET IS DESIGNED TO CONTROL THE RELEASE RATES FROM THE POND. IT SHALL BE DESIGNED IN A WAY TO MINIMIZE UNAUTHORIZED MODIFICATIONS WHICH AFFECT PROPER FUNCTION. A SIGN WITH A MINIMUM AREA OF 0.75 SQUARE FEET SHALL BE ATTACHED TO THE OUTLET OR POSTED NEARBY WITH THE FOLLOWING MESSAGE: "WARNING UNAUTHORIZED MODIFICATION OF THIS OUTLET IS A ZONING CODE VIOLATION"

SUMP PUMP DAYLIGHTS TO CURB  
INV. IN = 29.50

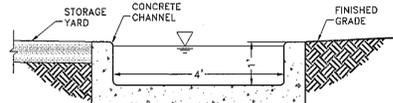
4X1" DRAINAGE CHANNEL  
CAPACITY = 24.67CFS

NEW 4" MANHOLE  
RIM = 5422.50  
INV. IN = 5418.91  
INV. OUT = 5418.61

EXISTING 22ND AVENUE DITCH  
OUTLET = 5418.50

ELEV = 5415.20

Section A-A  
SCALE: N.T.S.



DRAINAGE CHANNEL B-B  
N.T.S.

LIMITS OF INUNDATION IN THE EVENT THAT THE 22ND AVENUE DITCH OVERTOPS

**ENGINEERS CERTIFICATE:**

I HEREBY CERTIFY THAT THESE DRAWINGS WERE PREPARED BY ME AND/OR UNDER MY SUPERVISION AND THAT I AM A REGISTERED PROFESSIONAL ENGINEER UNDER THE STATUTES OF THE STATE OF COLORADO.

ROBERT D. COLOSIMO, P.E. #37327  
DATE 06/27/06

APPROVED FOR ONE YEAR FROM THIS DATE  
6-28-06

**OWNER:**  
 CMC GROUP, INC.  
 COLORADO CENTER, TOWER 1  
 2000 S. COLORADO BLVD. SUITE 10500  
 DENVER, CO 80222  
 CONTACT: GRAHAM BENES (303)741-4500

W.E. McManis, R.R. City Engineer  
 DATE 4/14/06  
 For J.E.W. DATE 6/29/06

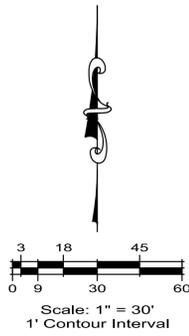
**R&R ENGINEERS-SURVEYORS, INC.**  
 710 WEST COLFAX AVENUE  
 DENVER, COLORADO 80204  
 PH 303-753-6730 FX 303-753-6568  
 WWW.RRENINEERS.COM

**PRELIMINARY DRAINAGE PLAN**  
 SITE: SPECIALTY WOOD PRODUCTS, TOWER CENTER FOR INDUSTRY FILING #2, L1, B EAST 22 AVE & TOWER ROAD AURORA, CO  
 FOR: CMC GROUP, INC. 2000 S. COLORADO BLVD. SUITE 10500 DENVER, CO 80222

**REVISIONS**

PRELIMINARY DRAINAGE 4/7/06
RESUBMIT 05/24/06
MYLARS 06/07/06

File Name	CM06002
Plot Date	6/7/2006
Date	03/30/06
Drawn	CD
Checked	RDC
Job No.	
CM06002	of 1



**LEGEND**

- (E) ELECTRICAL LINE
- (G) GAS LINE
- (SS) SANITARY SEWER LINE
- (T) TELEPHONE LINE
- (W) WATER LINE
- (E) CONTOUR LINE
- (P) CONTOUR LINE
- (P) BERM RIDGE LINE
- BASIN LINE
- (E) FIRE HYDRANT
- (E) MANHOLE
- DRAINAGE FLOW ARROW
- ← (P) EMERGENCY OVERTFLOW
- (E) EXISTING ITEMS
- (P) PROPOSED ITEMS
- △ DESIGN POINT

**BASIN DESIGNATION**

A	0.01	0.02	MINOR RUNOFF COEFFICIENT
A	0.05	0.10	MAJOR RUNOFF COEFFICIENT

BASIN AREA IN ACRES

**DETENTION POND DATA**

DETENTION POND TRIBUTARY AREA = 9.23 ACRES

MICROPOOL WSEL = 5424.20

WQCV = 0.198 ACRE-FEET; WQCV ELEVATION = 5427.15; DEPTH = 2.95'

EURV = 0.561 ACRE-FEET; EURV ELEVATION = 5429.19; DEPTH = 4.98'

100-YEAR+10EURV DETENTION VOLUME = 1.543 ACRE-FEET

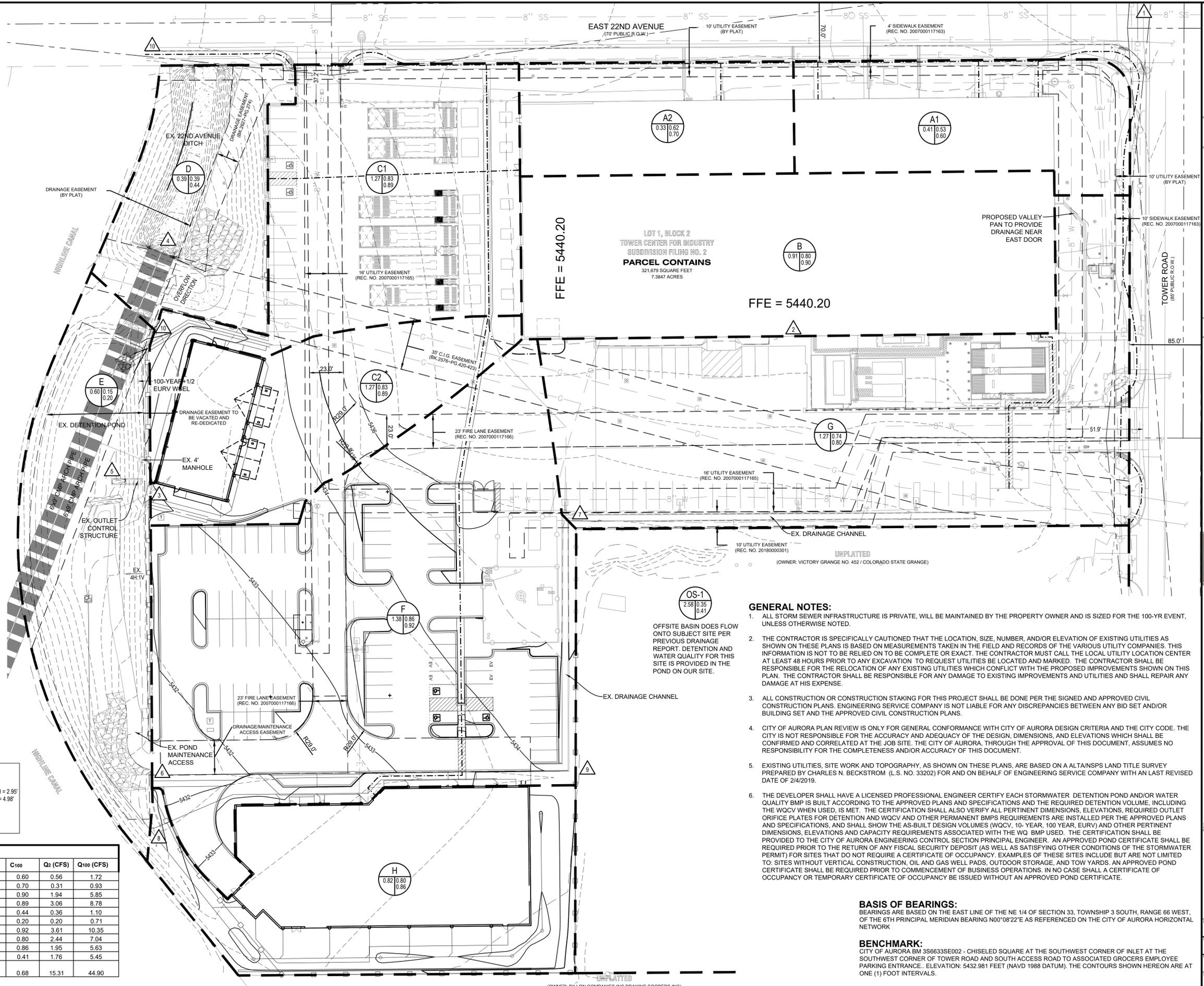
100-YEAR+10EURV WSEL = 5431.11; DEPTH = 6.91'

100-YEAR RELEASE = 6.1 CFS

SPILLWAY CREST EL = 5431.06

**SUMMARY RUNOFF TABLE**

D.P.	BASIN	I (%)	C <sub>2</sub>	C <sub>100</sub>	Q <sub>2</sub> (CFS)	Q <sub>100</sub> (CFS)
1	A1	53.10%	0.53	0.60	0.56	1.72
10	A2	65.63%	0.62	0.70	0.31	0.93
2	B	90.00%	0.80	0.90	1.94	5.85
3	C	95.04%	0.83	0.89	3.06	8.78
4	D	34.29%	0.39	0.44	0.36	1.10
5	E	2.00%	0.15	0.20	0.20	0.71
6	F	98.69%	0.86	0.92	3.61	10.35
7	G	82.05%	0.74	0.80	2.44	7.04
8	H	90.08%	0.80	0.86	1.95	5.63
9	OS-1	29.53%	0.35	0.41	1.76	5.45
Total Into Pond						
(All Except A1 and A2)		65.98%	0.62	0.68	15.31	44.90



- GENERAL NOTES:**
- ALL STORM SEWER INFRASTRUCTURE IS PRIVATE, WILL BE MAINTAINED BY THE PROPERTY OWNER AND IS SIZED FOR THE 100-YR EVENT, UNLESS OTHERWISE NOTED.
  - THE CONTRACTOR IS SPECIFICALLY CAUTIONED THAT THE LOCATION, SIZE, NUMBER, AND/OR ELEVATION OF EXISTING UTILITIES AS SHOWN ON THESE PLANS IS BASED ON MEASUREMENTS TAKEN IN THE FIELD AND RECORDS OF THE VARIOUS UTILITY COMPANIES. THIS INFORMATION IS NOT TO BE RELIED ON TO BE COMPLETE OR EXACT. THE CONTRACTOR MUST CALL THE LOCAL UTILITY LOCATION CENTER AT LEAST 48 HOURS PRIOR TO ANY EXCAVATION TO REQUEST UTILITIES BE LOCATED AND MARKED. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE RELOCATION OF ANY EXISTING UTILITIES WHICH CONFLICT WITH THE PROPOSED IMPROVEMENTS SHOWN ON THIS PLAN. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DAMAGE TO EXISTING IMPROVEMENTS AND UTILITIES AND SHALL REPAIR ANY DAMAGE AT HIS EXPENSE.
  - ALL CONSTRUCTION OR CONSTRUCTION STAKING FOR THIS PROJECT SHALL BE DONE PER THE SIGNED AND APPROVED CIVIL CONSTRUCTION PLANS. ENGINEERING SERVICE COMPANY IS NOT LIABLE FOR ANY DISCREPANCIES BETWEEN ANY BID SET AND/OR BUILDING SET AND THE APPROVED CIVIL CONSTRUCTION PLANS.
  - CITY OF AURORA PLAN REVIEW IS ONLY FOR GENERAL CONFORMANCE WITH CITY OF AURORA DESIGN CRITERIA AND THE CITY CODE. THE CITY IS NOT RESPONSIBLE FOR THE ACCURACY AND ADEQUACY OF THE DESIGN, DIMENSIONS, AND ELEVATIONS WHICH SHALL BE CONFIRMED AND CORRELATED AT THE JOB SITE. THE CITY OF AURORA, THROUGH THE APPROVAL OF THIS DOCUMENT, ASSUMES NO RESPONSIBILITY FOR THE COMPLETENESS AND/OR ACCURACY OF THIS DOCUMENT.
  - EXISTING UTILITIES, SITE WORK AND TOPOGRAPHY, AS SHOWN ON THESE PLANS, ARE BASED ON A LTA/NSPS LAND TITLE SURVEY PREPARED BY CHARLES N. BECKSTROM (L.S. NO. 33202) FOR AND ON BEHALF OF ENGINEERING SERVICE COMPANY WITH AN LAST REVISED DATE OF 2/4/2019.
  - THE DEVELOPER SHALL HAVE A LICENSED PROFESSIONAL ENGINEER CERTIFY EACH STORMWATER DETENTION POND AND/OR WATER QUALITY BMP IS BUILT ACCORDING TO THE APPROVED PLANS AND SPECIFICATIONS AND THE REQUIRED DETENTION VOLUME, INCLUDING THE WQCV WHEN USED, IS MET. THE CERTIFICATION SHALL ALSO VERIFY ALL PERTINENT DIMENSIONS, ELEVATIONS, REQUIRED OUTLET ORIFICE PLATES FOR DETENTION AND WQCV AND OTHER PERMANENT BMPS REQUIREMENTS ARE INSTALLED PER THE APPROVED PLANS AND SPECIFICATIONS, AND SHALL SHOW THE AS-BUILT DESIGN VOLUMES (WQCV, 10-YEAR, 100-YEAR, EURV) AND OTHER PERTINENT DIMENSIONS, ELEVATIONS AND CAPACITY REQUIREMENTS ASSOCIATED WITH THE WQ BMP USED. THE CERTIFICATION SHALL BE PROVIDED TO THE CITY OF AURORA ENGINEERING CONTROL SECTION PRINCIPAL ENGINEER. AN APPROVED POND CERTIFICATE SHALL BE REQUIRED PRIOR TO THE RETURN OF ANY FISCAL SECURITY DEPOSIT (AS WELL AS SATISFYING OTHER CONDITIONS OF THE STORMWATER PERMIT) FOR SITES THAT DO NOT REQUIRE A CERTIFICATE OF OCCUPANCY. EXAMPLES OF THESE SITES INCLUDE BUT ARE NOT LIMITED TO: SITES WITHOUT VERTICAL CONSTRUCTION, OIL AND GAS WELL PADS, OUTDOOR STORAGE, AND TOW YARDS. AN APPROVED POND CERTIFICATE SHALL BE REQUIRED PRIOR TO COMMENCEMENT OF BUSINESS OPERATIONS. IN NO CASE SHALL A CERTIFICATE OF OCCUPANCY OR TEMPORARY CERTIFICATE OF OCCUPANCY BE ISSUED WITHOUT AN APPROVED POND CERTIFICATE.

**BASIS OF BEARINGS:**  
 BEARINGS ARE BASED ON THE EAST LINE OF THE NE 1/4 OF SECTION 33, TOWNSHIP 3 SOUTH, RANGE 66 WEST, OF THE 6TH PRINCIPAL MERIDIAN BEARING N00°08'22"E AS REFERENCED ON THE CITY OF AURORA HORIZONTAL NETWORK

**BENCHMARK:**  
 CITY OF AURORA BM 356633SE02 - CHISELED SQUARE AT THE SOUTHWEST CORNER OF INLET AT THE SOUTHWEST CORNER OF TOWER ROAD AND SOUTH ACCESS ROAD TO ASSOCIATED GROCERS EMPLOYEE PARKING ENTRANCE. ELEVATION: 5432.981 FEET (NAVD 1988 DATUM). THE CONTOURS SHOWN HEREON ARE AT ONE (1) FOOT INTERVALS.

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 Aurora, Colorado 80014  
 P 303.337.1393  
 F 303.337.7481  
 T/F 1.877.273.0699  
 engineering@esvc.com

**PRELIMINARY DRAINAGE PLAN**  
 LOT 1, BLOCK 2, TOWER CENTER FOR INDUSTRY SUBDIVISION FILING NO. 2  
 T-MOBILE PH II SWITCH EXPANSION - CIVIL CONSTRUCTION PLANS  
 SITUATED IN THE NE 1/4 OF SECTION 33, T.3S., R.66W., OF THE 6TH P.M.  
 CITY OF AURORA, COUNTY OF ADAMS, STATE OF COLORADO

**SWSG CONSTRUCTION MANAGEMENT**  
 555 HERNDON PARKWAY, SUITE 280  
 HERNDON, VA 20170-5262  
 PHONE: (703) 471-8803  
 A.T.N. MARK KING

Prepared Under the Direct Supervision Of:  
 Chief M. Anderson, P.E. No. 5006  
 Registered Professional Engineer, State of Colorado  
 Designed by: [Signature] Engineering No.: E-20-013  
 Drawn by: CMA Date: 2021-02-08  
 Checked by: CMA Scale: Horiz.: 7/77 Vert.: N.A.  
 Sheet No.: 1 OF 1